# COMPARISON OF THE HYDRAULIC CALCULATIONS TABLES NETWORKS SEWERAGE <br> Kalyuzhniy A.P., Zubricheva L.L., Kryvenko O.O. Poltava National Technical Yuri Kondratyuk University, Poltava e-mail: anatoliy1975@mail.ru 


#### Abstract

The article has analyses the calculations of the networks sewerage, made the calculated tables of different authors. There has been shown the possibility of reducing the capital costs for materials and construction of networks sewerage. There have been proposed a more economically expedient table.


Keywords: sewerage, hydraulic calculation, wastewater, diameter, pipes.
General problem statement. Wastewaters affect the sanitary condition of cities and towns. A feature of the sewerage network is: gravity movement mode. For normal operation of the sewage network must correctly calculations by which determine the size of the pipe, slope, speed and filling pipe. The hydraulic calculations are considered by some of scientific works [1-2, 4-6, 8, 9].

The purpose of this analysis is to search for possible technical solutions that allow the rational designing of and reconstruction of sewage system.

Main part. A gravity sewer network is design to incomplete filling. The purpose of hydraulic calculation is the selection of pipe diameters, slopes and defining the network marks the trays and the depth of laying pipe. In practice, the hydraulic calculation of sewer networks is performed using the calculation tables.

The appearances of modern materials pipes and standard documents require changes in hydraulic calculation table [1, 2]. The issue of hydraulic calculation received much attention, but not compared the different methods and tables for calculation. There are no accurate recommendations for the use of certain tables. Therefore there is a need to study this problem.

For practical calculations take the town, this situated on the territory of Ukraine (Fig.1).


Fig. 1. Sewerage networks plan of the town
Town consists of 2-regions that differ in the number of inhabitants and the degree of improvement [3]. Sewage system is separate. Domestic and industrial
waste water discharged into a sewer network. The main collector transports wastewater to the main sewage pumping station (SPS), then to the Wastewater Treatment Plant (WTP). Tracing the sewage network is made on the plan of the town.

Hydraulic calculation of sewerage networks is made using Microsoft Excel. All sewer networks are calculated three times at different tables [4, 5, 6]. As a result of hydraulic calculation pipe diameters, slopes and defining the network marks the trays and the depth of laying pipe are identified for the sewer network of the town.

Comparison variants calculation on the example of the main collector 0-9 shown in Table 1, where diameters (d), slopes of networks (i) and the average depth laying of the pipe (h).

Table 1-Comparison variants calculation

| number <br> plots | [6] |  |  | [4] |  |  | [5] |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\mathbf{d , m m}$ | $\mathbf{i}$ | $\mathbf{h , m}$ | $\mathbf{d , m m}$ | $\mathbf{i}$ | $\mathbf{h , m}$ | $\mathbf{d , m m}$ | $\mathbf{i}$ | $\mathbf{h , m}$ |
| $0-1$ | 250 | 4 | 1.922 | 250 | 5 | 1.922 | 200 | 5 | 2.128 |
| $1-2$ | 315 | 3,2 | 3.718 | 350 | 3 | 4.016 | 315 | 3.2 | 4.177 |
| $2-3$ | 400 | 2,5 | 5.070 | 400 | 3 | 5.253 | 450 | 2.2 | 5.524 |
| $3-4$ | 500 | 2 | 5.593 | 500 | 2 | 5.756 | 500 | 2 | 5.940 |
| $4-5$ | 500 | 2 | 3.173 | 500 | 2 | 3.415 | 560 | 1.8 | 3.547 |
| $5-6$ | 500 | 2 | 3.903 | 600 | 2 | 4.302 | 630 | 1.6 | 4.260 |
| $6-7$ | 630 | 1,6 | 3.394 | 700 | 1.5 | 3.689 | 630 | 1.6 | 3.672 |
| $7-8$ | 800 | 1,3 | 5.727 | 800 | 1.3 | 5.828 | 800 | 1.2 | 6.264 |
| $8-9$ | 800 | 1,3 | 4.910 | 900 | 1.1 | 5.089 | 900 | 1.1 | 5.524 |

The optimum diameters of the pipe with respect to the calculation of the three options are greyed.

When using tables [5] for small discharge diameter on the plot $0-1$ received the least in comparison with [4] and [6]. However, with increasing discharge of waste water in the plots $2-3,4-5,5-6$ and $8-9$ diameters are greatest. The
maximum diameter pipes reached 900 mm , and the depth of laying the network in point 8 reached 6.34 m .

When using tables [4] laying depth is slightly smaller, but in some plots (0-1, $1-2,5-6,6-7,8-9)$ piping diameters are necessary larger than the other two calculations.

When using tables [6] laying depth is less than in the previous two cases, the system will be fully function at smaller diameters. This, in turn, affects the economic feasibility of using the system, calculated according to these tables.

Polyethylene pipes are much more resistant to wear than concrete, ferroconcrete and plastic pipes. So now is to give preference to polyethylene pipes. When laying pipes from polymeric materials hydraulic calculation should be carried out on tables [6].

## Conclusions

1. Table [4] should be used when the drainage system being projected with ceramic, concrete or of ferroconcrete pipes.
2. Table. [5] are calculated using formulas of uniform motion of liquid with using a roughness coefficient for plastic pipes. Thus the tables should be used when designing the network of sewage pipes of plastic
3. The diameters and the depth laying of the pipe in the calculation of the tables [6] are lower than in the previous two calculations. And the system is functioning normally.
4. The tables should be taken when calculating the sewage system to be built out of corrugated plastic pipe company «KORSIS» and it affects their economic advantage.

## References:

1. Yevropeyskiy standart EN13476-1 «Sistema plastmassovykh truboprovodov dlya podzemnoy prokladki beznapornoy kanalizatsii i drenazha. Sistema
dvukhsloynykh gofrirovannykh trub iz neplastifitsirovannogo polivinilkhlorida (PVKH-U), polipropilena (PP) i polietilena (PE) ».
2. DBN V. 2.5. - 75: 2013 Kanalízatsíya. Zovníshní merezhí ta sporudi. - Kiïv .: MRRB ta ZHKG Ukraïni, 2013. - 210s.
3. DBN 360-92 ** «Místobuduvannya. Planuvannya í zabudova mís'kikh í síl's'kikh poselen'» / Derzhbud Ukraïni. -K., 2002;
4. Lukinikh A.A., Lukinikh N.A., Tablitsy dlya gidravlicheskogo rascheta kanalizatsionnykh setey i dyukerov po formule akad. N.N. Pavlovskogo / 4-ye izd, dop. M .; Stroyizdat, 1974. -156 s.
5. Karelin YA. A., Tablitsy dlya gidravlicheskogo rascheta kanalizatsionnykh setey iz plastmassovykh trub kruglogo secheniya: spravochnoye posobiye / M .: Stroyizdat, 1986. - 53 s.
6. Korsis. Beznapornaya i livnevaya kanalizatsiya: tekhnicheskoye rukovodstvo.

- K: / Gruppa poli plastik, 2011. - 44 s .

7. Rekomendatsii dlya otsenki izmeneniya stoimosti, trudoyemkosti i materialoyemkosti stroitel'stva v proyektakh promyshlennykh zdaniy i sooruzheniy / TSNII. - M .: Stroyizdat, 1989. - 607 s .
8. Kanalizatsiya naselennykh mest i promyshlennykh predpriyatiy / N.I.Likhachev, I.I.Larin, S.A.Khaskin i dr .: pod obshch. red V.N.Samokhina -2-ye izd., pere rab. i dop. - M .: Stroyizdat, 1981. - 639 s. - (Spravochnik proyektirovshchika).
9. Voronov YU.V. Osobennosti gidravlicheskogo rascheta polimernykh truboprovodov Korsis / YU.V.Voronov, Ye.A.Pugachev // Polimernyye truby. 2007. - №2 (3). - S. 42 - 45.
